Water Sensitive Urban Design (WSUD) Strategy Report

Industrial Redevelopment

54-68 Ferndell Street

South Granville NSW 2142

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Introduction

Sparks & Partners Consulting Engineers have been engaged by Dexus Funds Management to undertake the concept civil engineering design for a proposed development at 54-68 Ferndell St, South Granville. The engineering services include the design and documentation of the stormwater drainage infrastructure and finished surface levels for the proposed development.

Cumberland Council is the approval authority and has recently been formed through the amalgamations of Auburn and Holroyd Councils, and the redefining of Local Government Area (LGA) boundaries with Parramatta Council. The site itself previously fell under Parramatta Council prior to the redefining of the LGA boundaries. This Water Sensitive Urban Design (WSUD) Strategy report has been prepared by taking into consideration the objectives and controls under the Parramatta City Council Stormwater Disposal Policy and Development Control Plan (DCP). In response to this requirement Sparks and Partners has undertaken modelling of the proposed WSUD measures and prepared this report to demonstrate that the proposed industrial redevelopment identifies and incorporates water conservation and stormwater management measures into its design and operation.

The current design incorporates on-site detention for all areas of the site not affected by overland flow from upstream properties. Refer to the Overland flow Flood Study Report also submitted with this development application (DA).

This report revision addressed comments on stormwater issues, and revised architectural layouts resulting from council's initial review of the development application.

Existing Site

The site has been previously used for pharmaceutical manufacturing and has several warehouses, office buildings, storage areas, carparking, circulation roadways and associated infrastructure such as storage tanks and pipelines. The existing site buildings have recently been demolished in preparation for redevelopment of the site. A south-west portion of the site is a biodiversity area that is undeveloped and contains an earth drainage channel. There are two 1800mm diameter stormwater drainage pipelines that run through the site from the channel in the biodiversity area to Ferndell Street and further downstream toward Duck River located to the east. The site is bounded by 74 Ferndell Street and 6, 8 & 10 Ferngrove Place on the southern boundary, Ferndell Street on the eastern boundary, 46 Ferndell Street and 10 Straits Avenue along the northern boundary and Campbell Hill Pioneer Reserve on the western boundary.

Proposed Development

The proposed development occupies a total area of 93,000m² and consists of four warehouse buildings, café, offices hardstand, car park pavement areas and landscaping. The buildings occupy 58,565m², the pavement area occupies 30,845m² and the landscaping occupies 3,590m². There is an existing undeveloped area in the south west corner of the site that is to remain largely unchanged, with regrading of the drainage channel eastern bank and works around an existing easement pipe headwall to occur. Detailed architectural plans have been prepared by Nettleton Tribe architects and are to be read in conjunction with this report.

Overland Flow

Existing twin 1800mm diameter pipes below the site have been assessed to determine the volume and extents of any associated overland flow. Currently, a large portion of the site is inundated during the 100 year Average Recurrence Interval (ARI) storm and runoff greater than the pipe capacity is conveyed through the site as overland flow. The proposed design eliminates the overland flow by



providing an overflow pipe to convey flows through the site that are in excess of the easement pipe capacity during the 100ARI storm whilst assuming 50% blockage at the headwall. The level of overland flow during the 100ARI storm and hydraulic grade line (HGL) of flows within the easement pipes has been accounted for in the design of the On-Site Detention (OSD) and stormwater drainage system. Upstream catchments that drain through the site bypass the OSD catchments. Drawings with the extents of overland flow are located in Appendix A. For further detailed information regarding the overland flow refer to the Sparks and Partners Overland Flow Flood Study Report.

Water Sensitive Urban Design (WSUD)

The objective of WSUD is to provide a strategy that brings together the different aspects of the water cycle as a whole rather than an ad hoc approach to water management. This includes the management aspects of freshwater, wastewater and stormwater. The following WSUD strategies have been considered and addressed for the proposed development:

- Employ an integrated water collection and recycling system for capturing and recycling roofwater:
- Control the quality of stormwater that is disposed from the site;
- Control the quantity of stormwater that is discharged for the site.

To demonstrate the above concept stormwater drainage plans and associated details have been prepared along with detailed modelling using the MUSIC software package. The concept stormwater drainage plans detail the location of the water management infrastructure including pits, pipes, Enviropods, Stormfilter Cartridges, OSD and rainwater tanks, and are included in Appendix A for reference.

Rainwater Reuse

Through the reuse of collected roofwater for non-potable reuse the proposed demand on potable water resources is reduced. The proposed development will capture roof water from part of each warehouse building roof area. This collected roofwater will be conveyed to a tank for storage and reuse throughout the development. Re-use purposes will primarily include irrigation uses. A water balance of the proposed reuse system is to be completed at the detailed design stage to model the effectiveness and efficiency of the system based upon the demand for irrigation.

Stormwater Quality

To ensure the quality of stormwater leaving the site is acceptable and meets WSUD requirements specific water quality treatment measures are to be employed. These treatment measures are to treat the collected stormwater runoff prior to discharge to the council drainage system. The treatment measures consist of Ocean Protect Oceanguards (formerly Stormwater 360 Enviropods) and PSorb Stormfilter Cartridges. The combination of these measures provides a treatment train approach to the treatment of stormwater runoff with Oceanguards providing primary treatment and the PSorb Stormfilters providing secondary treatment.

This development falls within the newly formed Cumberland Council in an area that was previously in the Parramatta LGA. The WSUD measures have been implemented to meet targets in accordance with the Parramatta City Council (PCC) DCP 2011.

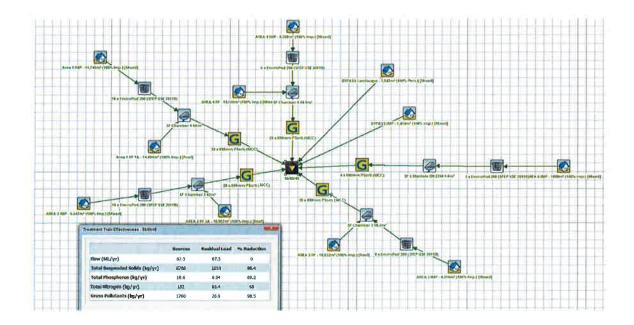
Modelling of the proposed treatment measures has been undertaken using the MUSIC software package version 6. The pre-loaded source nodes in Parramatta MUSIC-link produce the same pollutant loads for all source surfaces, an approach which differs from the NSW MUSIC modelling guidelines. Previous advice from PCC engineers has allowed an approach to use pre-loaded Blacktown City Council nodes, which differentiate the pollutant loads from different sources such as roof and ground surfaces. This approach provides a more accurate model of the site pollutant loads. A catchment plan



has been prepared to differentiate the surface types and confirm which catchments are draining to which treatment measures. The WSUD catchment plan is located in Appendix A for reference. The modelling results of the water quality achieved for the site is detailed in Table 1 – MUSIC Model Results below, along with a figure of the prepared model. The results show that the proposed WSUD measures and treatment train approach ensures the development will meet the required pollutant reduction targets. Refer to the submitted MUSIC model for verification.

	Source Load	Residual Load	% Reduction Achieved	PCC % Reduction Requirement	Compliance with PCC Requirement
Total Suspended Solids (kg/yr)	8760	1020	88.4	85	Υ
Total Phosphorus (kg/yr)	19.6	6.04	69.2	60	Υ
Total Nitrogen (kg/yr)	152	83.4	45.0	45	Υ
Gross Pollutants (kg/yr)	1760	26.9	98.5	90	Y

Table 1 - MUSIC Model Results





Stormwater Quantity

The proposed development requires the implementation of on-site detention (OSD) as per the PCC Engineering Guide for Development to control stormwater discharge from the site. The OSD has been designed in accordance with Parramatta City Council's Stormwater Disposal Policy and the Upper Parramatta River Catchment Trust (UPRCT) Handbook Revision 4. In accordance with previous advice from council, the undeveloped biodiversity area which is affected by overland flow, has not been included in the OSD catchment. Utilising surface ponding or partial surface ponding as OSD was considered during the design phase, however, is unfeasible due to the nature of operations within the proposed development, therefore the developed areas of the site will drain to below ground OSD tanks. The site area has been divided into four catchments each with their own OSD tanks and discharge points, with an area bypassing the OSD. The OSD tanks discharge to the easement and overflow pipes that dissect the site. Due to the existing easement pipes and large size of the site, each OSD has a separate discharge point. An OSD catchment plan of the proposed development is included in Appendix A. The OSD calculations allow for approximately 6.6% bypass within each catchment to account for the bypass area along the Ferndell Street frontage. Table 2 below provides a summary of the catchments within the site.

OSD	Catchment Area (m²)	Percentage Bypassing OSD (%)	Total OSD Volume Required (m³)
1	26,236	7.1	1,365
2	17,456	7.0	908
3	22,940	6.7	1,193
4	19,848	6.6	1,033

Table 2 - Site OSD Catchment Summary

PCC requires OSD to be designed using the UPRCT handbook. The UPRCT OSD summary sheet has been utilised to determine the required volume and discharge for the proposed development. The site is situated within the Duck River catchment and Table 3 contains the site storage requirement (SSR) and site reference discharge (SRD) rates that have been used in the design.

OSD	Primary Outlet	Secondary Outlet	Total SSR	Notes
	SRD (L/s/ha)	SRD (L/s/ha)	(m3/ha)	
1	40	150	520	HED Configuration
2	40	150	520	Drowned Outlet – Non HED Configuration
3	40	150	520	HED Configuration
4	40	150	520	HED Configuration

Table 3 - OSD Design Parameters

The OSD summary sheets are located in the Appendix B for review and the completed UPRCT OSD checklist is located in Appendix C for review.

Maintenance and Monitoring

To ensure the continued efficient and correct operation of the proposed integrated water management infrastructure a 'maintenance and monitoring schedule' is included in the Appendix D of this plan. The schedule details the frequency of inspections, what is to be inspected and what rectifications to make if required for the water management infrastructure located within the proposed development. The schedule is to be implemented upon commissioning of the water management infrastructure and remain in place for the life of the development; with all records kept on site for inspection should the approval authority deem it necessary.



Conclusion

Based on the concept stormwater drainage plans & MUSIC modeling results, it is demonstrated that the principles of water sensitive urban design have been incorporated into the design and operation of the proposed development at 54-68 Ferndell Street South Granville in accordance with PCC Stormwater Disposal Policy. It is demonstrated that the proposed development will achieve reductions in potable water import by capturing rainwater on site and reusing this for non-potable uses including irrigation, achieves runoff pollution reduction targets set by council, and employs OSD for the control of stormwater discharge from the site in accordance with targets set by council. It is also demonstrated that the proposed development will continue to operate effectively and efficiently through the implementation and use of a monitoring and maintenance schedule, ensuring the integrity of the system is maintained.



Appendix A – Concept Drainage Plans

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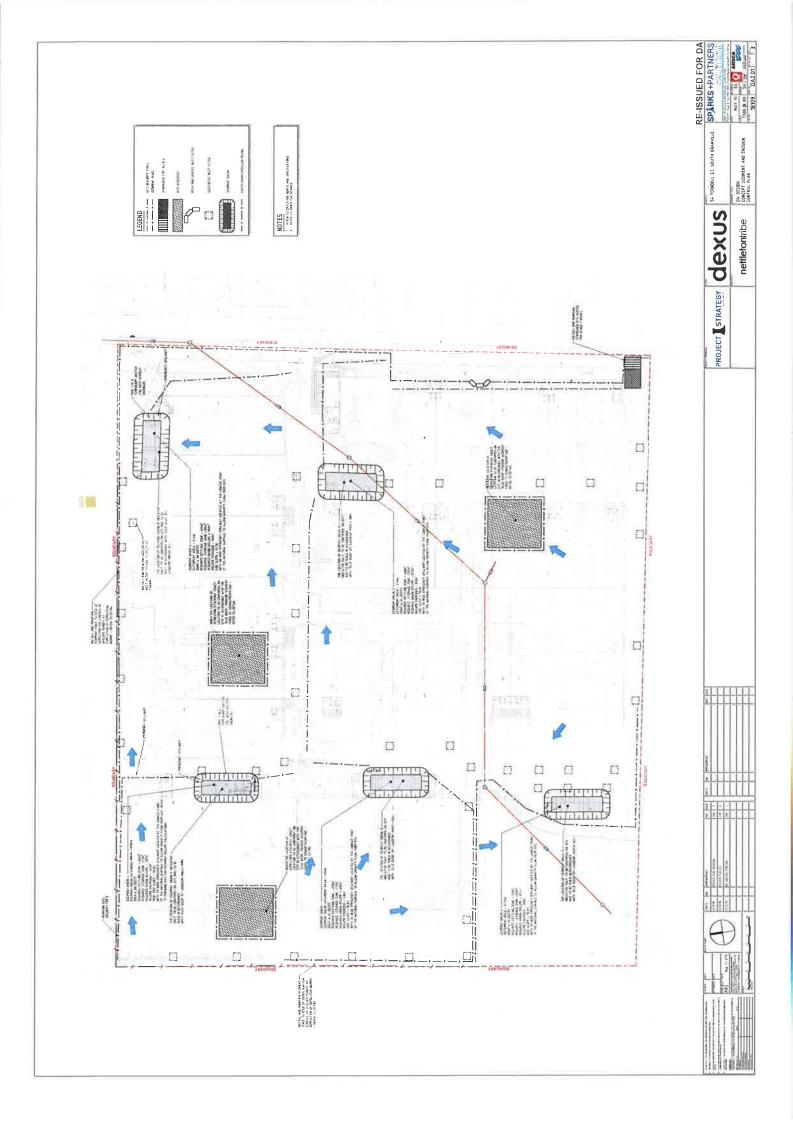
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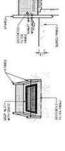
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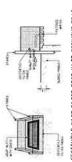
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DA DESIGN COVER SHEET, NOTES, LOCALITY PLAN AND DRAWING SCHEDULE









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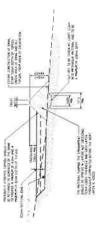
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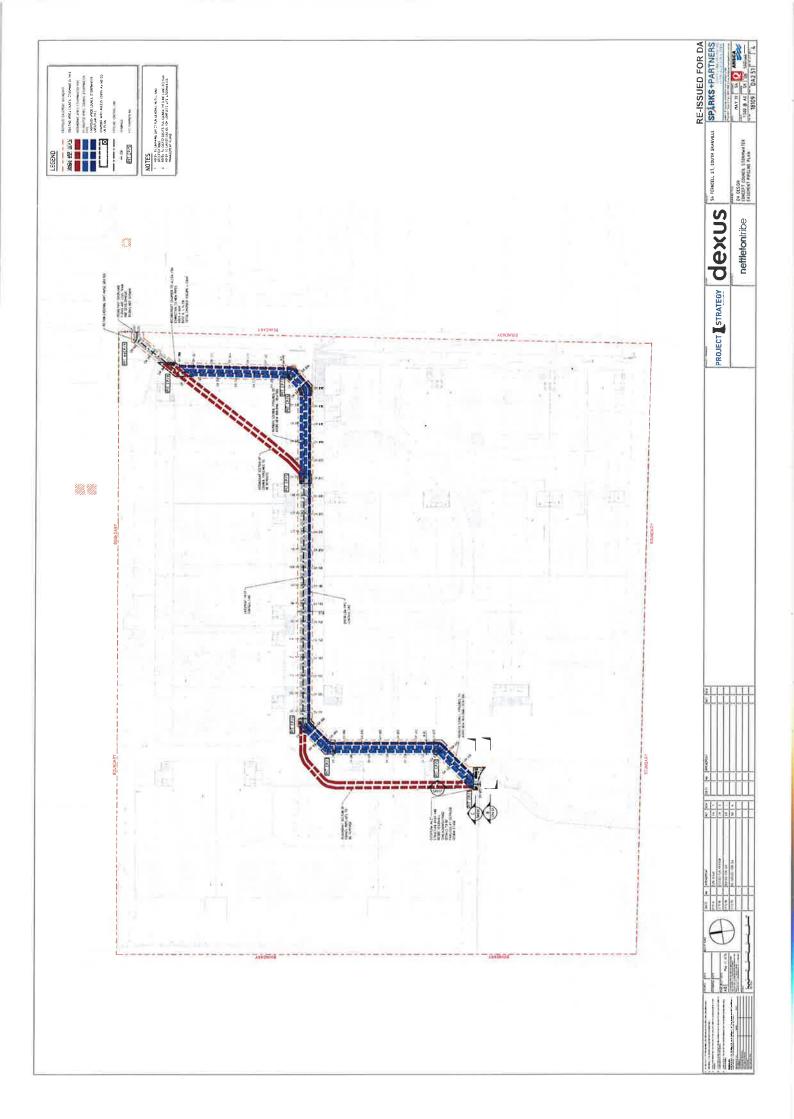
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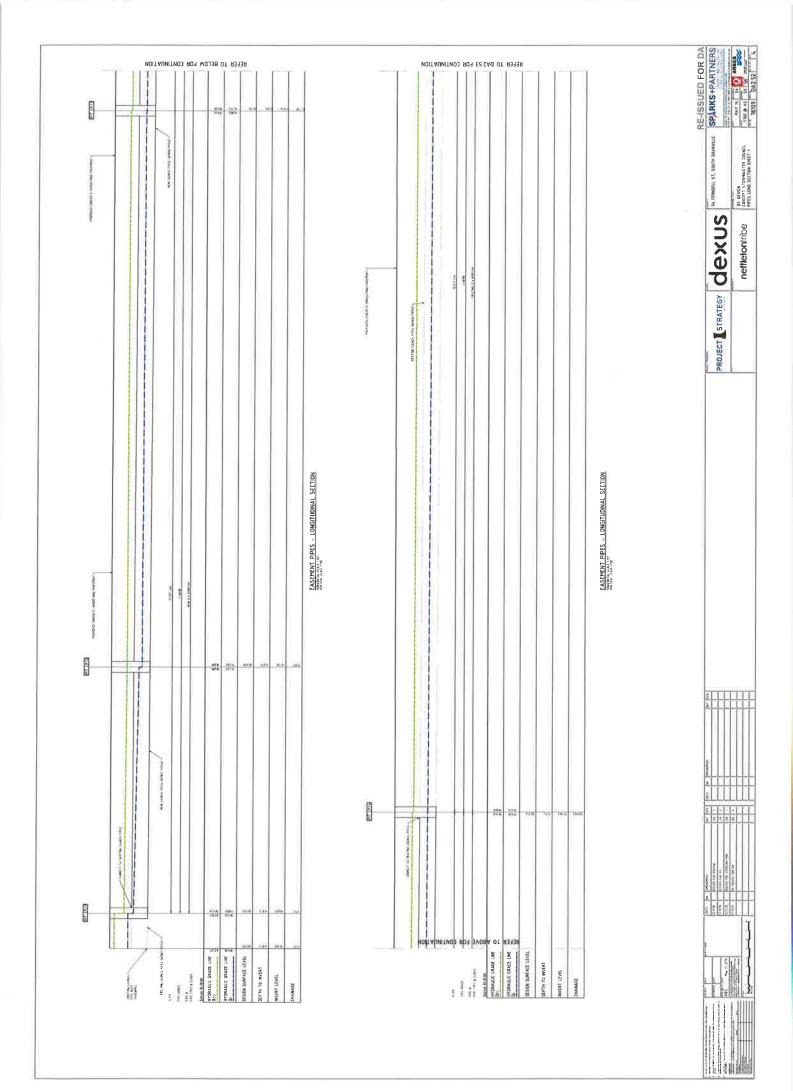


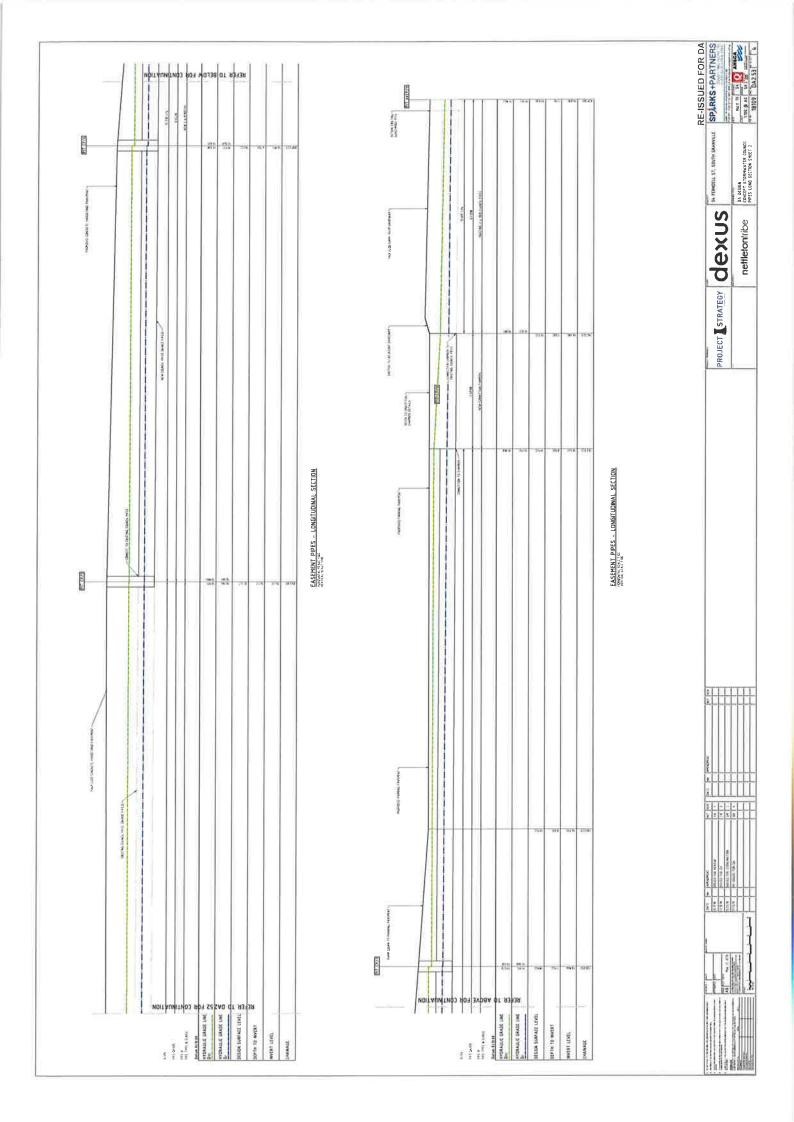
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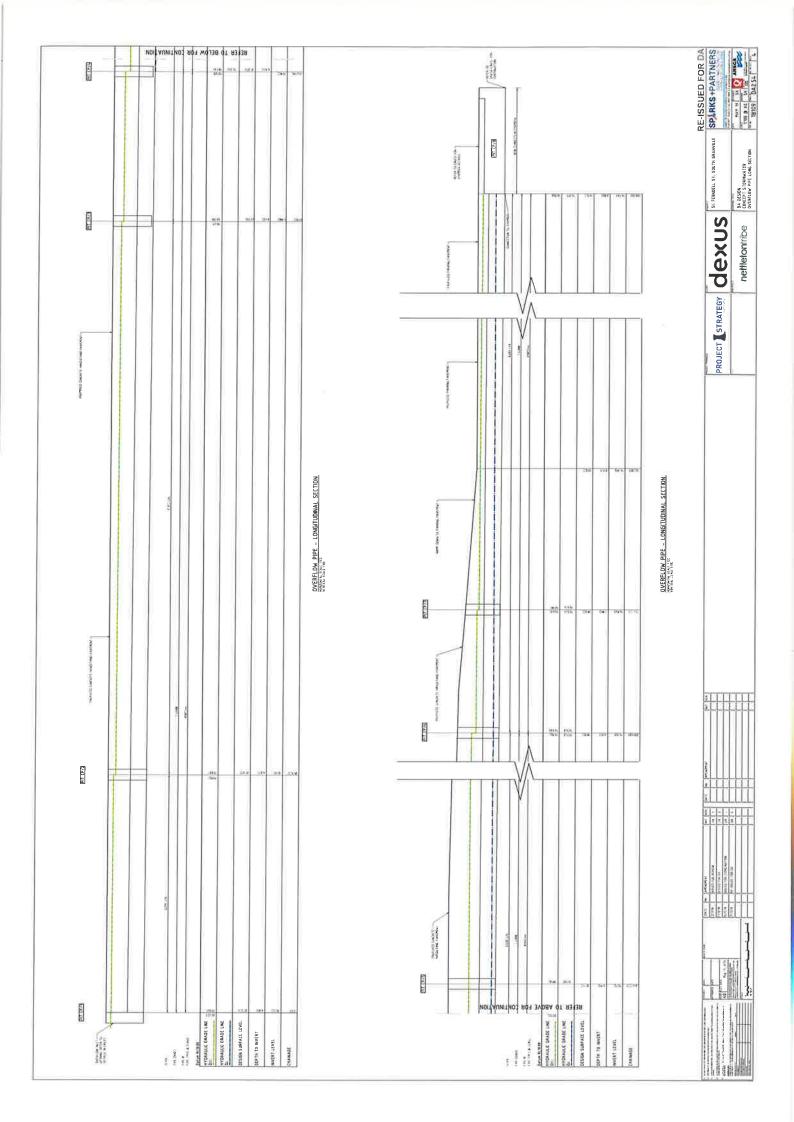
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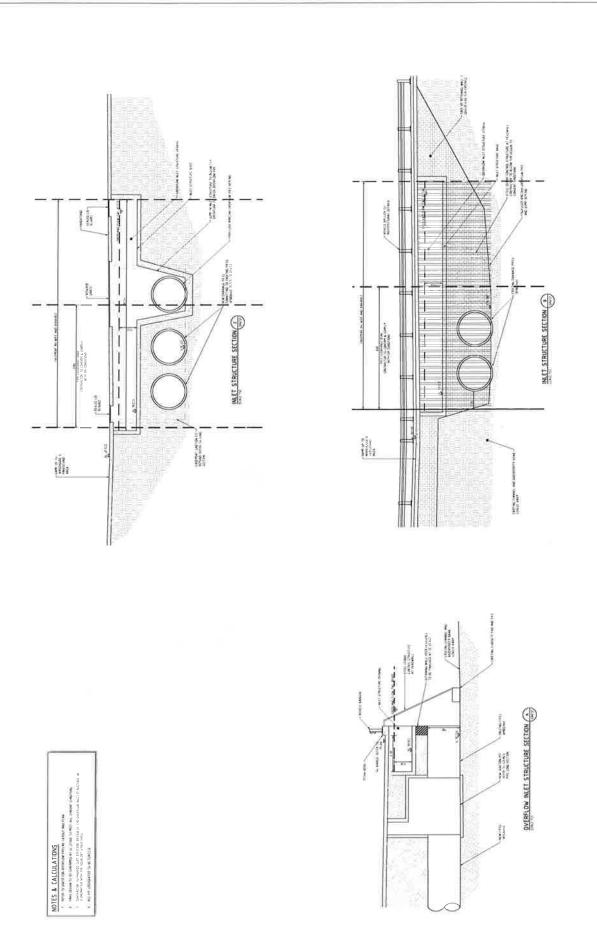
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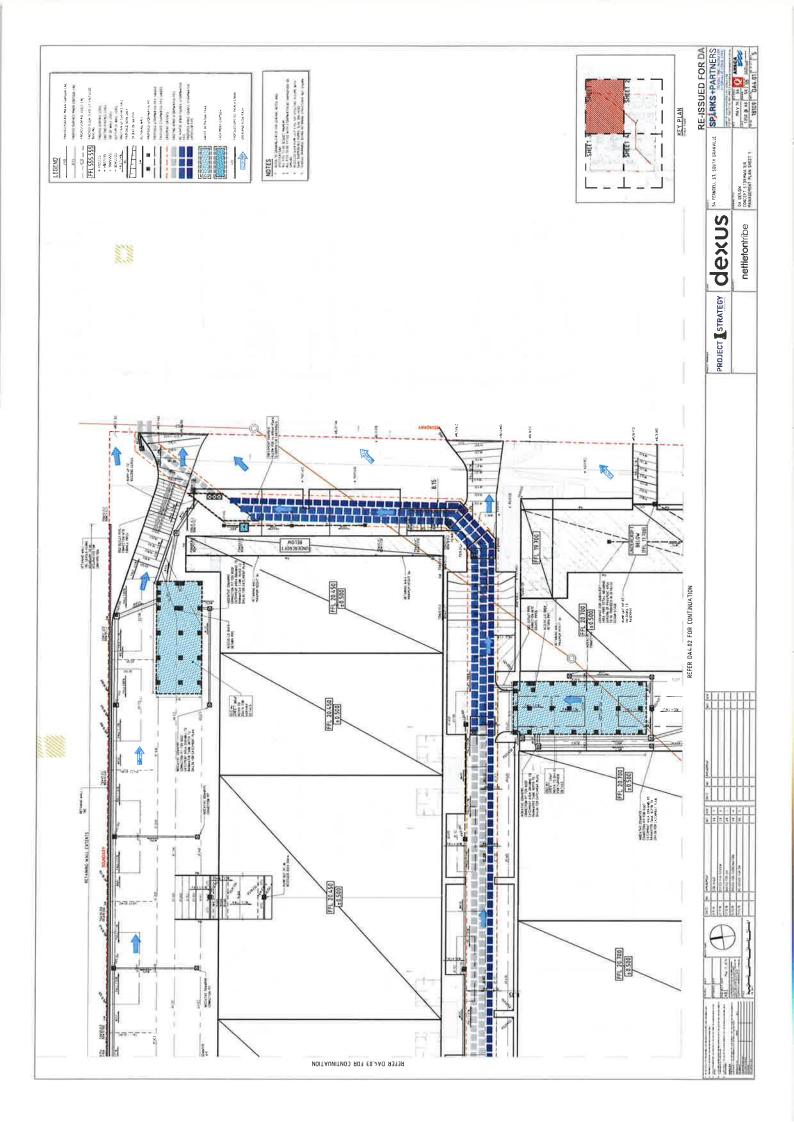


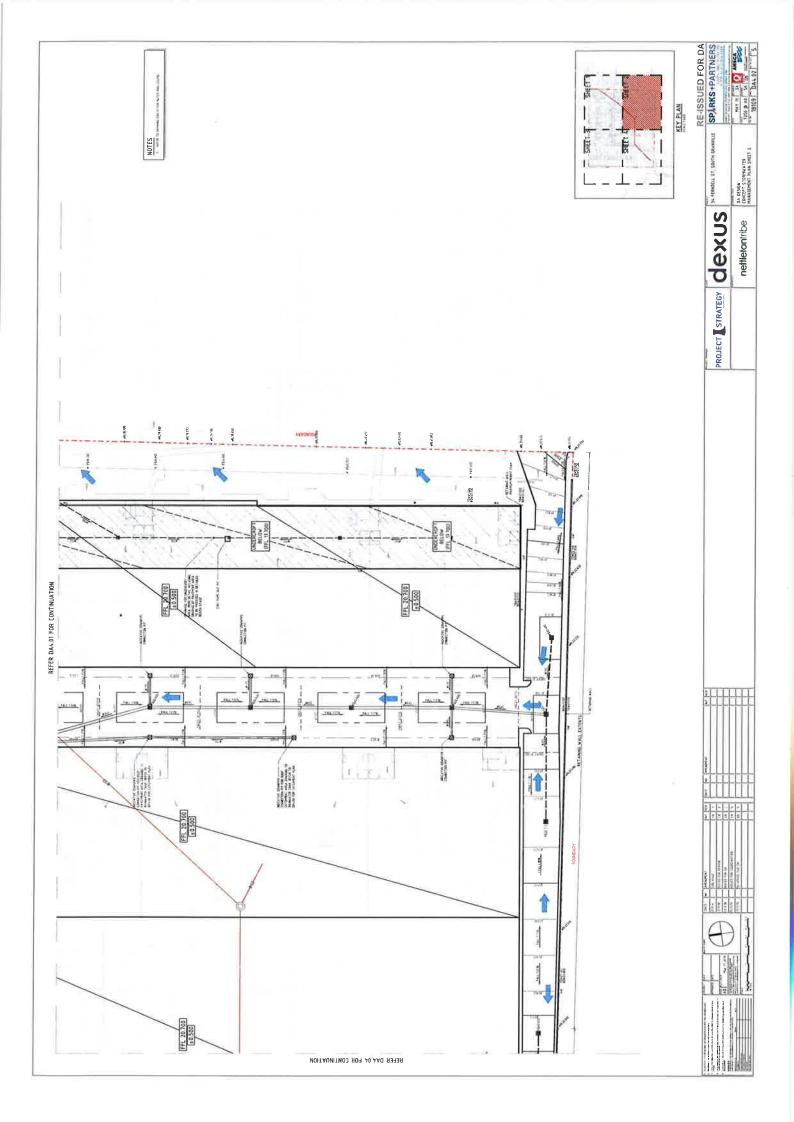
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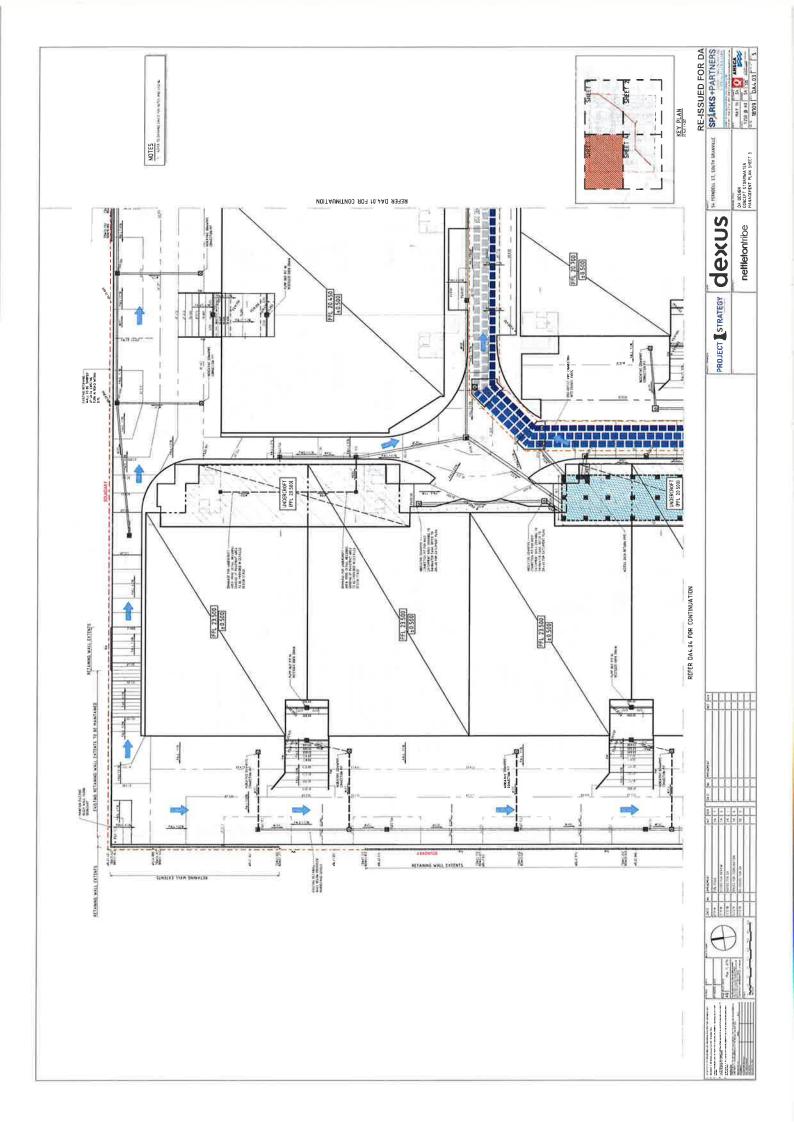
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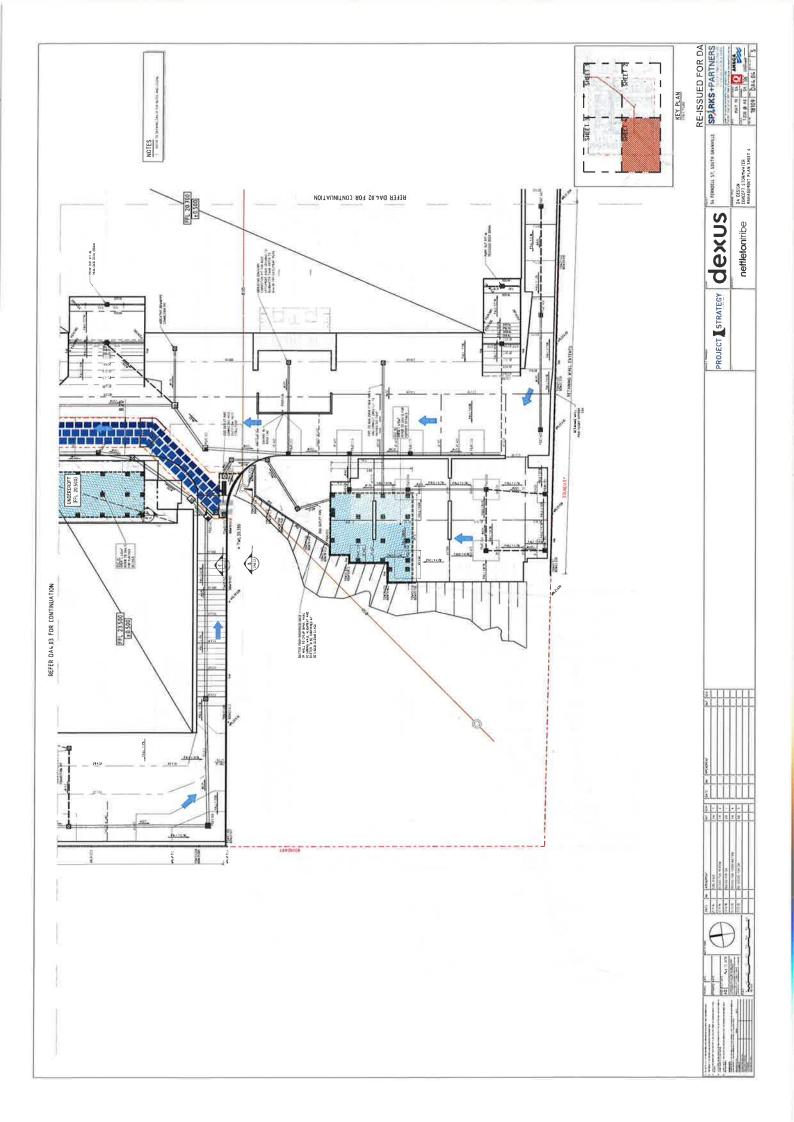
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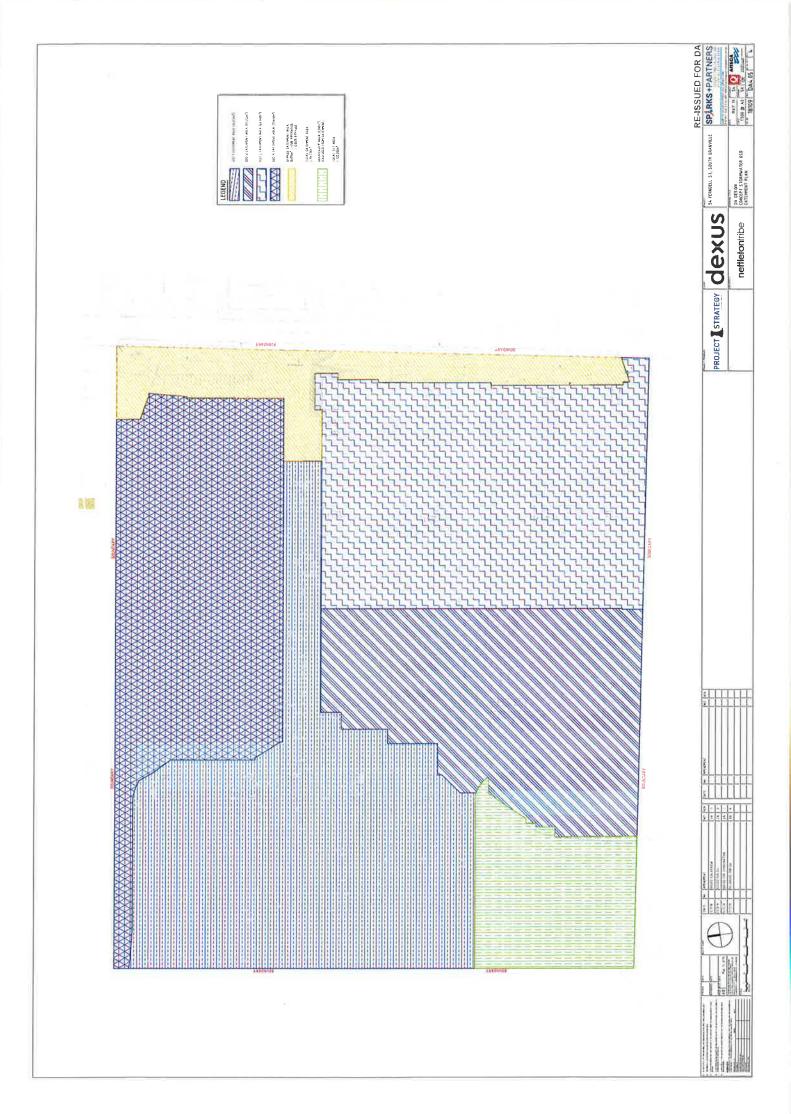
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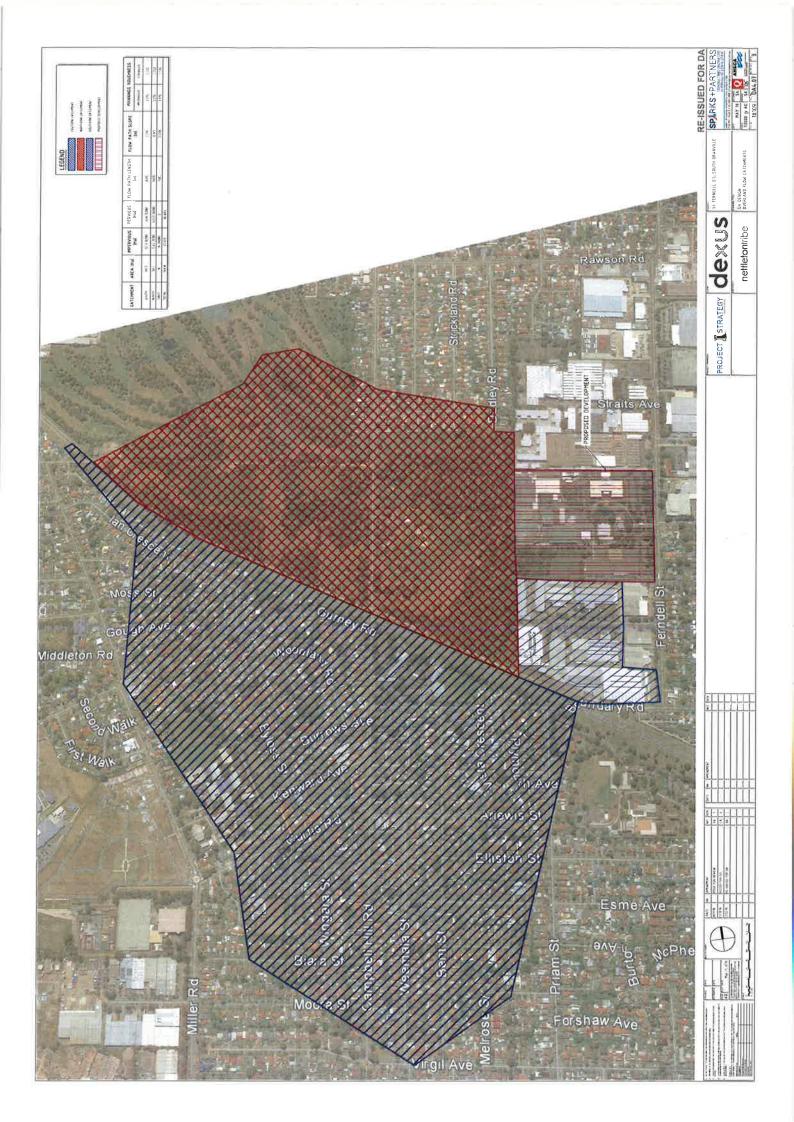


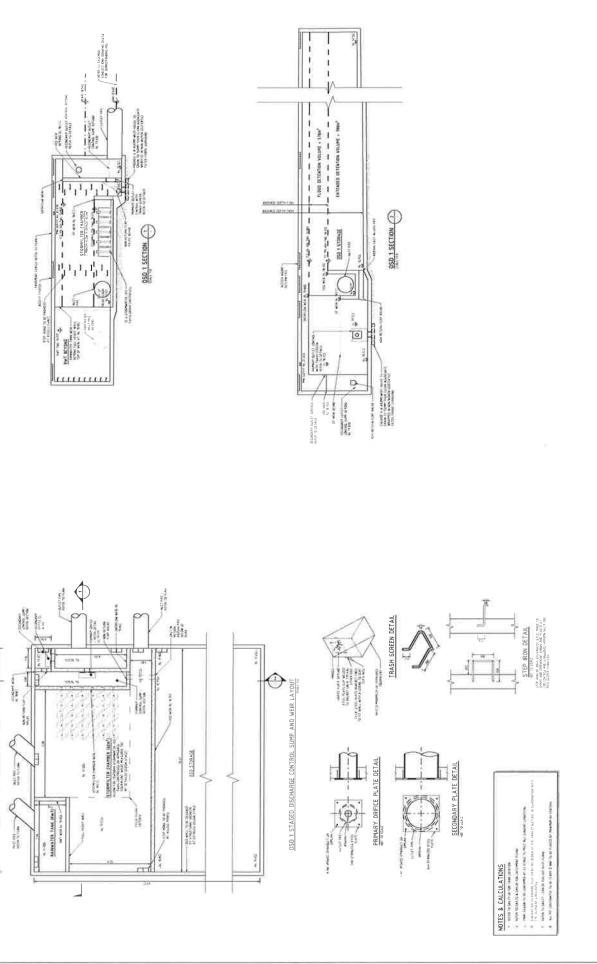




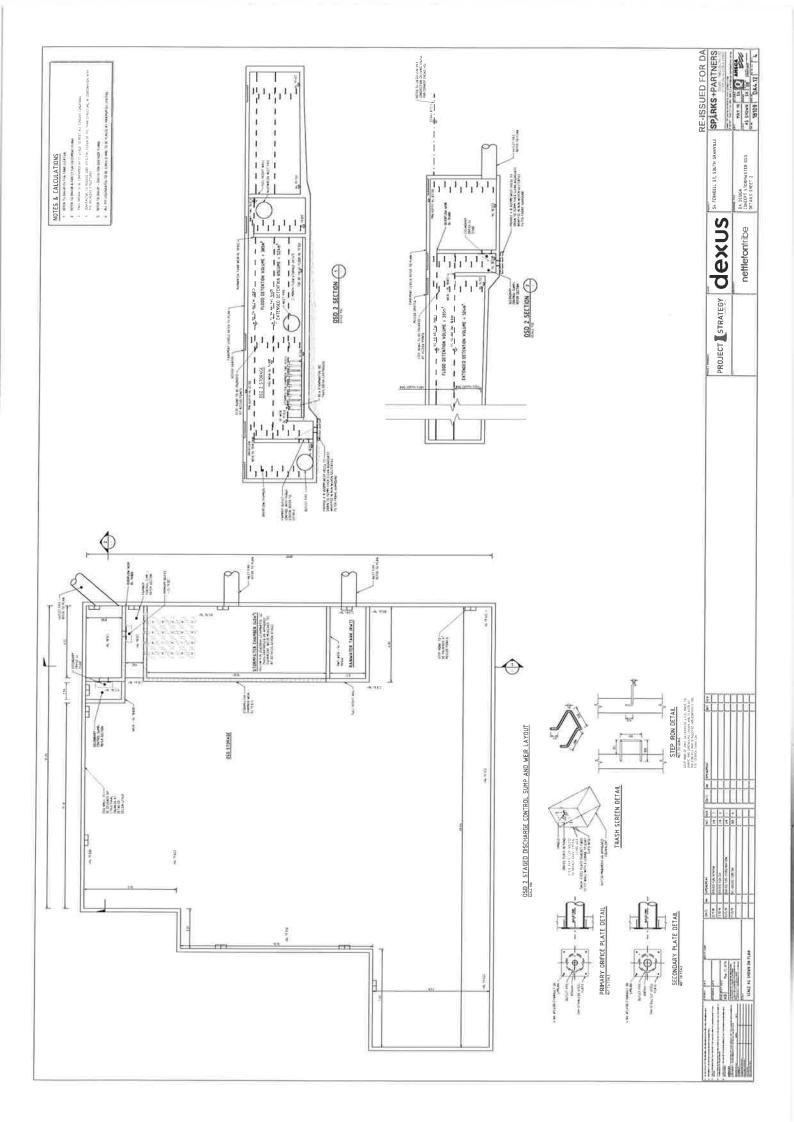


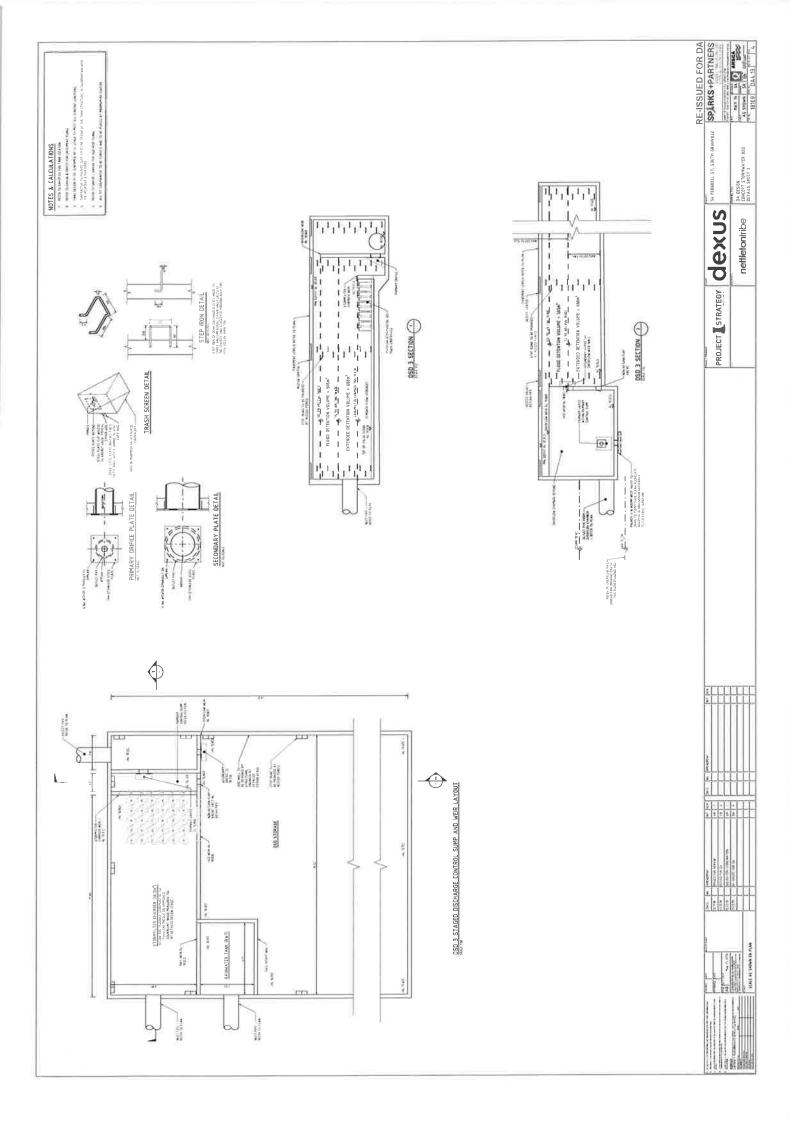


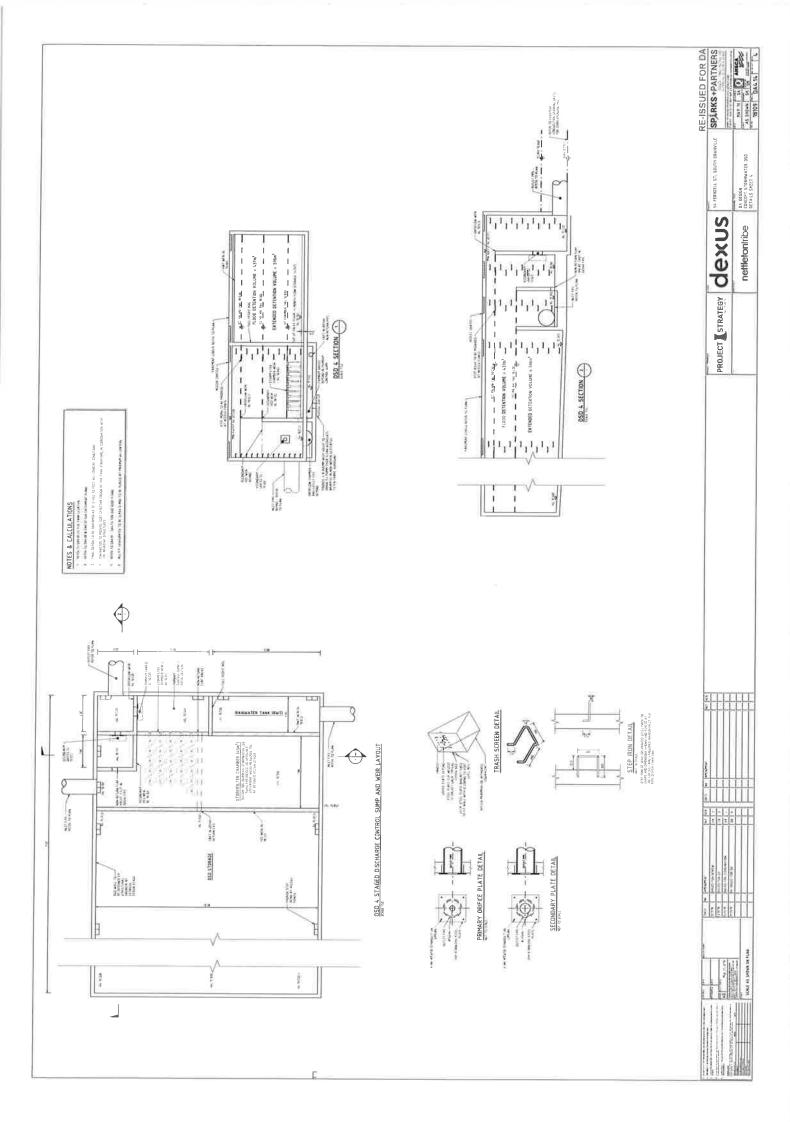


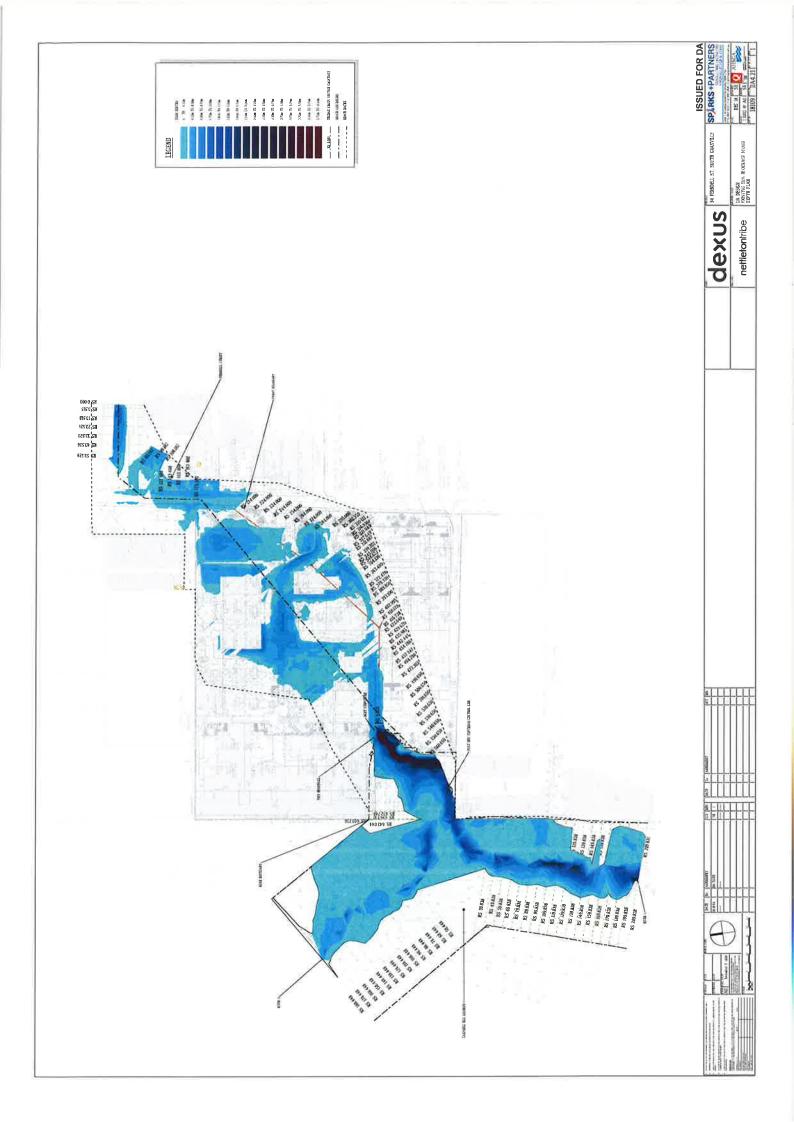


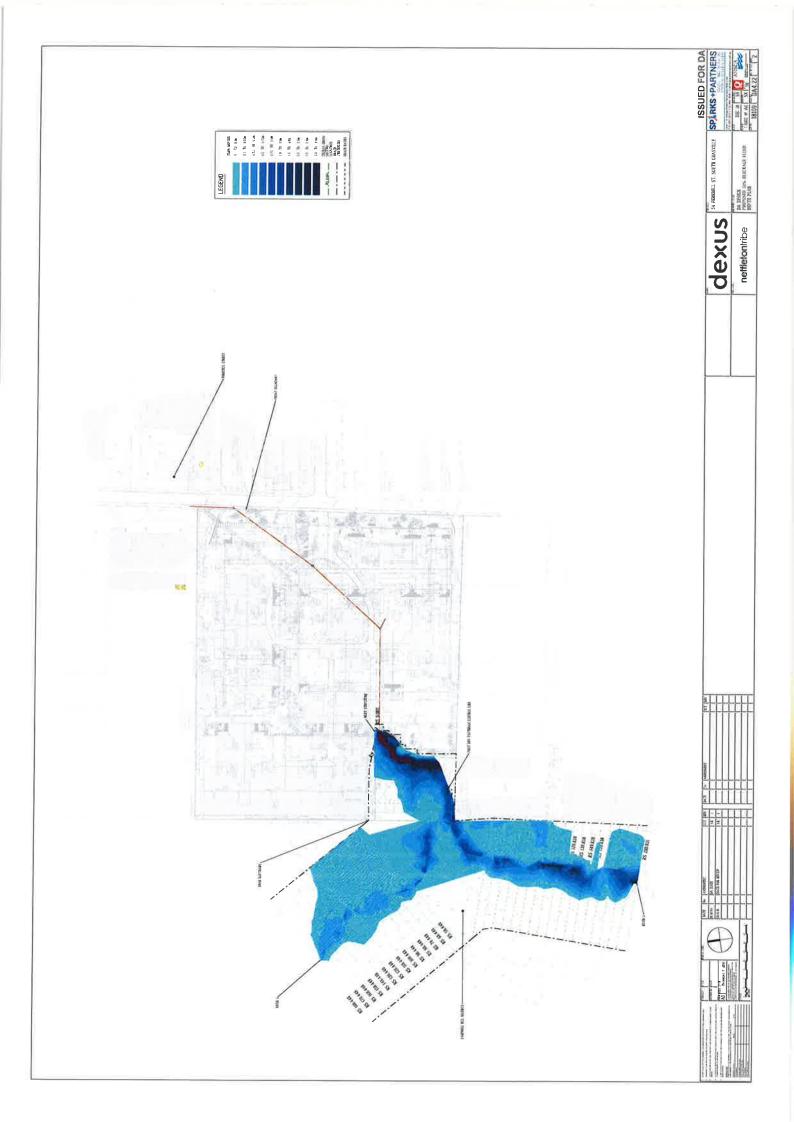
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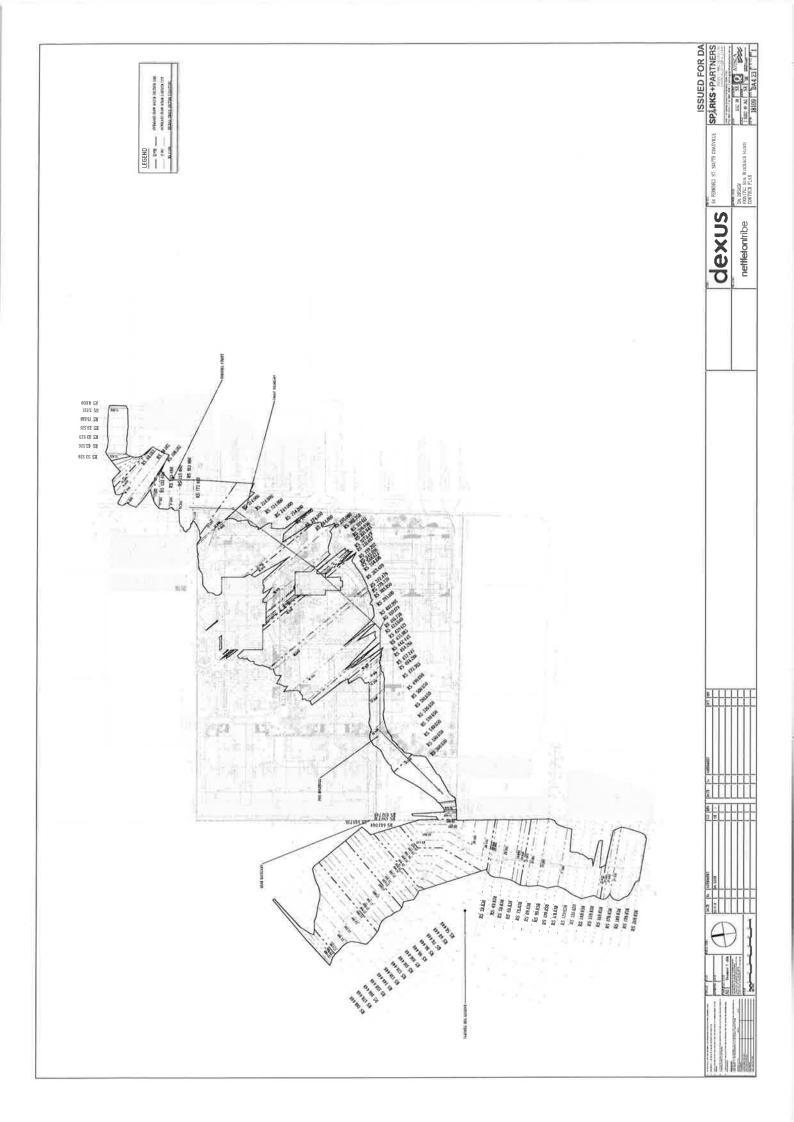
















Appendix B – UPRCT OSD Calculation Summary

On-Site Detention Calculation Sheet for Carctments outside Upper Parramatta River Catchment HED Secondary Outlet

			D Sec	condary	Out	iet		
Project:	DA OSD 1							
Site Address	54-68 FERNDELL	STREET S	OUTH G	RANVILLE				
Job No:	18109							
Designer:	ВВ							
Telephone:	02 9891 5033							
	1		S	ite Data				100
OSD Area:		Duck Rive	r					
L.G.A		Parramatta	a City Co	uncil				
Site Area		2.6236	ha	26,236	m ²			
Total Roof Area		1.4495	ha	14,495	m ²			
Area of Site draining t	o OSD Storage	2.54	ha	25,400		Satisfactory		
Residual Site Area (Lo	ot Area - Roof Area)	1.174	ha					
Area Bypassing Stora	ge	0.0836	ha					
Area Bypassing / Resi	idual Site Area	7.1%				Satisfactory		30% Ma
No. of Dwellings on Si	te	1				Satisfactory		
Site Area per Dwelling	•	2.624	ha					
Roof Area per Dwellin	g	1,450	ha					
		D.	nois Of	SD Paran	97099			
		Extended I			ierens		Detar#-	
Basic SSR Vols	Ext Detention Storage		m ³ /ha	п		Total Starona	Detention 520	m³/ha
Basic SRDs	Primary Outlet		L/s/ha			Total Storage Secondary Oullet		L/s/ha
2200 81180	Filmary Odile	40	L/S/IIa			Secondary Outlet	150	Lisina
			OSD T	ank Byp	ass			
Residual Lot Capture i	n OSD Tank	93%						
Adjusted SRDs		38	L/s/ha				136	L/s/ha
			OSD (Calculatio	ns			
		Extended [ו			Detention	
Basic SSR Volume	Ext Detention Storage		m ³			Total Storage	1364.27	m ³
Total Rainwater Tank (Credits	0.00	m ³				0.00	m ³
Storage Volume						Total	1364.27	m ³
Storage Volume	Ext Detention Storage		m ³			Flood Detention Storage	577.19	m ³
OSD Discharges	Primary Outlet	99.34	L/s			Secondary Outlet	356.18	L/s
DI . (T.)		Western I						
RL of Top Water Level	-	18.750	m				19.800	m
RL of Orifice Centre-lin	e r	17.100	m			r	18.700	m
Number of Orifices		1	_			ļ	1	
Estimated Downstream		16.82	1.5 yr Af				18.55	100 yr ARI
Downstream FL - RL of		-0.28	Satisfa	ctory		Satisfactory	-0.15	m
Design Head to Orifice		1.650	m			Ext Detn Storage - RL Orifice	0.050	m
Calculated Orifice Diam	neter	192	mm	Satisfacto	У	Satisfactory	874	mm
	O	verflow V	Veir &	Freeboar	d Cal	culation	-	1, 15 0
RL of Minimum Habitab							20.450	m
RL of Minimum Garage	Floor Level						20.450	m
Length of Overflow Wei							6.00	m
Site Runoff Coefficient						Parramatta City Council	0.75	
Storm Intensity (5 min 1	00 yr ARI)						206	mm/h
Peak Flow over Weir							1090,1	L/s
Depth of Flow over Wei							231	mm
						Satisfactory	446	
Freeboard to Habitable Freeboard to Garage FI						Satisfactory	419 419	mm

THE MAINTAINE NO.	C. III. Burney	ie same size rainwater tar	UNITED AND ASSESSMENT OF THE PARTY OF THE PA	Min	Max
% of Roof draining to Rainwater Tank	0.0%		Satisfactory	0.0%	100%
Total Rainwater Tank Volume	0.00	kL	Minimum 0.0 kL		
Min Volume that triggers Top-up	0.00	kL	Note - Min Vol in Tank	< 10% Total T	ank Vol
Total Tank Vol - Min Top-up Vol	0.00	kL			
		Dedicated Airspace			
Dedicated Airspace	0.00	kL	Satisfactory		
	Extended I	Detention		Detention	
Dedicated Airspace Credit	0.00	kL		0.00	kL
Maximum Tank PSD	40	L/s/ha			
Maximum Tank Discharge	0.0	L/s			
Maximum Head to Centre of Tank Orifice	0.000	m	No Dedicated Airspace	Э	
Calculated Orifice Diameter	0	mm	No Dedicated Airspac	e	
		Dynamic Airspace		0.0 ± 0	
Maximum Dynamic Storage (Nett Vol)	0.00	kL	Controls minimum % l	Roof to Rainwa	ater Tank
Daily Demand on Rainwater Tank	0.657	kL/d	Satisfactory		
Dynamic Airspace at start of Storm	0.00	kL			
	Extended I	Detention		Detention	
Dynamic Airspace Credit	0.00	kL		0.00	kL
Combined Rainwater Tank Credit	0.00	kL		0.00	kL
Maximum Rainwater Tank Credit	0.00	kL		0.00	kL
Rainwater Tank Credit per Dwelling	0.00	kL		0.00	kL
Rainwater Tank Credit for the Site	0.00	m ³		0.00	m ³

Signature:	Date:
Signature.	Dute.

On-Site Detention Calculation Sheet for

Catchments outside Upper Parramatta River Catchment

Non-HED Secondary Outlet (Due to Elevated Downstream 100 yr ARI Flood Level)

	(Due to				_			
Project:	DA - OSD 2							
Site Address	54-68 FERNDELL	STREET S	OUTH GRA	NVILLE				
Job No:	18109							
Designer:	SK							
Telephone:	02 9891 5033							
			Site	Data	1, "			
OSD Area:		Duck river						
L.G.A		Parramatta	City Counc					
Site Area		1.7456	ha	17,456	m ²			
Total Roof Area		1.0905		10,905	m ²			
Area of Site draining to	OSD Storage	1.7	ha	17,000	m ²	Satisfactory		
Residual Site Area (Lo	t Area - Roof Area)	0.655	ha					
Area Bypassing Storag	je	0.0456	ha					
Area Bypassing / Resid	dual Site Area	7.0%				Satisfactory		30% Ma
No. of Dwellings on Sit	e	1				Satisfactory		
Site Area per Dwelling		1.746	ha					
Roof Area per Dwelling)	1.091	ha					
		Ba	sic OSD	Paran	inter	ra		-
		Extended [-		-	•	Detention	
Basic SSR Vols	Ext Detention Storage		m³/ha			Total Storage (1.14 x SSR _{THED})	520	m ³ /ha
Basic SRDs	Primary Oullet	40	L/s/ha			Secondary Outlet	150	L/s/ha
								20,112
			OSD Tan	k Byp	ass			
Residual Lot Capture in	1 OSD Tank	93%	OSD Tan	k Byp	ass			
Residual Lot Capture in	n OSD Tank		OSD Tan	k Byp	ass		136	L/s/ha
· ·	n OSD Tank	93% 38	L/s/ha				136	L/s/ha
· ·	n OSD Tank	93% 38	Us/ha				136	L/s/ha
Adjusted SRDs	n OSD Tank	93% 38 Extended D	Us/ha OSD Calco				136	
Adjusted SRDs Basic SSR Volume	Ext Detention Storage	93% 38 Extended D 523.68	Us/ha OSD Calc Detention m ³			Total Storage		m ³
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C	Ext Detention Storage	93% 38 Extended D	Us/ha OSD Calco			Total Storage	Detention	m³ m³
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume	Ext Detention Storage Credits	93% 38 Extended D 523.68 0.00	OSD Calconding m ³ m ³			Total Storage Total	Detention 907.71	m ³ m ³ m ³
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume	Ext Detention Storage	93% 38 Extended D 523.68 0.00	Us/ha OSD Calc Detention m ³				Detention 907.71 0.00	m³ m³
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume	Ext Detention Storage Credits	93% 38 Extended D 523.68 0.00	OSD Calconding m ³ m ³			Total	Detention 907.71 0.00 907.71	m ³ m ³ m ³
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges	Ext Detention Storage Credits Ext Detention Storage Primary Outlet	93% 38 Extended E 523.68 0.00 523.68 66.18	OSD Calconomics of the control of th			Total Flood Detention Storage	Detention 907.71 0.00 907.71 384.03 237.54	m³ m³ m³ m³
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage	93% 38 Extended D 523.68 0.00 523.68 66.18	OSD Calconomics of the control of th			Total Flood Detention Storage	Detention 907.71 0.00 907.71 384.03 237.54	m ³ m ³ m ³ L/s
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage	93% 38 Extended D 523.68 0.00 523.68 66.18	OSD Calcontion m³ m³ L/s			Total Flood Detention Storage	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000	m³ m³ m³ m³ L/s
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000	OSD Calino Detention m³ m³ L/s			Total Flood Detention Storage	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000	m³ m³ m³ d³
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000	OSD Calo Detention m³ m³ L/s L/s	culatio		Total Flood Detention Storage Secondary Outlet	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000	m³ m³ m³ m³ L/s
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30	OSD Call Detention m³ m³ L/s 1.5 yr ARI Satisfactor	culatio		Total Flood Detention Storage Secondary Outlet	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000 1 20.03 3.03	m³ m³ m³ L/s
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of Design Head to Orifice	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage e Flood Level Orifice Cente-line Centre	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m	culatio	ns	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice	Detention 907.71 0.00 907.71 384.03 237.64 19.800 17.000 1 20.03 3.03 2.800	m³ m³ m³ m³ L/s
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifice Estimated Downstream Downstream FL - RL of Design Head to Orifice	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage e Flood Level Orifice Cente-line Centre	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m	culatio	ns	Total Flood Detention Storage Secondary Outlet	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000 1 20.03 3.03	m³ m³ m³ L/s
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of Design Head to Orifice	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line Centre eter	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800 154	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m mm Sa	culatio	nns ·	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice	Detention 907.71 0.00 907.71 384.03 237.64 19.800 17.000 1 20.03 3.03 2.800	m³ m³ m³ m³ L/s
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of Design Head to Orifice Calculated Orifice Diam	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line Centre eter	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800 154	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m mm Sa	culatio	nns ·	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice Satisfactory	Detention 907.71 0.00 907.71 384.03 237.64 19.800 17.000 1 20.03 3.03 2.800	m³ m³ m³ m³ L/s
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line Centre eter	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800 154	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m mm Sa	culatio	nns ·	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice Satisfactory	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000 1 20.03 3.03 2.800 261	m³ m³ m³ m³ L/s m m 100 yr ARI
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of Design Head to Orifice Calculated Orifice Diam RL of Minimum Habitab RL of Minimum Garage	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line Centre eter Primary Outlet	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800 154	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m mm Sa	culatio	nns ·	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice Satisfactory	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000 1 20.03 3.03 2.800 261	m³ m³ m³ L/s m m 100 yr ARI
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of Design Head to Orifice Calculated Orifice Diam RL of Minimum Habitab RL of Minimum Garage Length of Overflow Weil Site Runoff Coefficient	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line Centre eter le Floor Level Floor Level	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800 154	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m mm Sa	culatio	nns ·	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice Satisfactory	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000 1 20.03 3.03 2.800 261	m³ m³ m³ m³ L/s m m m mm
Adjusted SRDs Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of Design Head to Orifice Calculated Orifice Diam RL of Minimum Habitab RL of Minimum Garage Length of Overflow Weil Site Runoff Coefficient Storm Intensity (5 min 1	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line Centre eter le Floor Level Floor Level	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800 154	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m mm Sa	culatio	nns ·	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice Satisfactory	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000 1 20.03 3.03 2.800 261 20.800 6.00 0.75 206	m³ m³ m³ m³ L/s L/s m m m m m m m
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of Design Head to Orifice Calculated Orifice Diam RL of Minimum Habitab RL of Minimum Garage Length of Overflow Weir Site Runoff Coefficient Storm Intensity (5 min 1 Peak Flow over Weir	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line Centre eter Primary Outlet Orifice Cente-line Orifice Cente-line Orifice Cente-line	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800 154	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m mm Sa	culatio	nns ·	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice Satisfactory	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000 1 20.03 3.03 2.800 261 20.800 6.00 0.75 206 729.6	m³ m³ m³ m³ L/s m m m m m m
Basic SSR Volume Total Rainwater Tank C Storage Volume Storage Volume OSD Discharges RL of Top Water Level RL of Orifice Centre-line Number of Orifices Estimated Downstream Downstream FL - RL of Design Head to Orifice Calculated Orifice Diam	Ext Detention Storage Credits Ext Detention Storage Primary Outlet of Storage Flood Level Orifice Cente-line Centre eter Primary Outlet Orifice Cente-line Orifice Cente-line Orifice Cente-line Orifice Cente-line	93% 38 Extended D 523.68 0.00 623.68 66.18 18.800 17.000 1 16.30 -0.70 1.800 154	OSD Cali Detention m³ m³ L/s 1.5 yr ARI Satisfactor m mm Sa	culatio	nns ·	Total Flood Detention Storage Secondary Outlet Adopt non-HED Outlet TWL Detn Storage - RL Orifice Satisfactory	Detention 907.71 0.00 907.71 384.03 237.54 19.800 17.000 1 20.03 3.03 2.800 261 20.800 6.00 0.75 206	m³ m³ m³ m³ L/s L/s m m m m m m m

ine calculations a	Saume Wat ti	ie same aize famwater ta	ik is installed on each dwelling	Min	Max
% of Roof draining to Rainwater Tank	0.0%		Satisfactory	0.0%	100%
Total Rainwater Tank Volume	0.00	kL	Minimum 0.0 kL		
Min Volume that triggers Top-up	0.00	kL	Note - MIn Vol in Tank < 1	0% Total Ta	nk Vol
Total Tank Vol - Min Top-up Vol	0.00	kĹ			
and the second		Dedicated Airspace			75.7
Dedicated Airspace	0.00	kL	Satisfactory		
	Extended I	Detention		Detention	
Dedicated Airspace Credit	0.00	kL		0.00	kL
Maximum Tank PSD	40	⊔/s/ha			
Maximum Tank Discharge	0.0	⊔s			
Maximum Head to Centre of Tank Orifice	0.000	m	No Dedicated Airspace		
Calculated Orifice Diameter	0	mm	No Dedicated Airspace		
		Dynamic Airspace			
Maximum Dynamic Storage (Nett Vol)	0.00	kL	Controls minimum % Roo	f to Rainwa	ter Tank
Daily Demand on Rainwater Tank	0.657	kL/d	Satisfactory		
Dynamic Airspace at start of Storm	0.00	kL			
	Extended (Detention		Detention	
Dynamic Airspace Credit	0.00	kL		0.00	kL
Combined Rainwater Tank Credit	0.00	kL		0.00	kL
Maximum Rainwater Tank Credit	0.00	kL		0.00	kL
Rainwater Tank Credit per Dwelling	0.00	kL		0.00	kL
Rainwater Tank Credit for the Site	0.00	m ³		0.00	m ³

Signature:	Date:	

On-Site Detention Calculation Sheet for Carctments outside Upper Parramatta River Catchment HED Secondary Outlet

7.1		HE	ບ Sec	ondary	Outl	et		
Project:	DA - OSD3							
Site Address	54-68 FERNDELL	STREET S	OUTH G	RANVILLE				
Job No:	18109							
Designer:	ВВ							
Telephone:	02 9891 5033							
			Si	te Data				
OSD Area:		Duck River	r					
L.G.A		Parramatta	City Cou					
Site Area		2.294	ha	22,940				
Total Roof Area		1.8622	ha		m ²			
Area of Site draining to	_	2.265	ha	22,650	m²	Satisfactory		
Residual Site Area (Lo		0.432	ha					
Area Bypassing Storag		0.029	ha					
Area Bypassing / Resi		6.7%				Satisfactory		30% Ma
No. of Dwellings on Si	te	1				Satisfactory		
Site Area per Dwelling		2.294	ha					
Roof Area per Dwelling	3	1.862	ha					
		Ba	sic OS	D Param	eters			
		Extended I	Detention				Detention	
Basic SSR Vols	Ext Detention Storage	300	m³/ha			Total Storage	520	m³/ha
Basic SRDs	Primary Outlet	40	L/s/ha			Secondary Outlet	150	L/s/ha
			OSD T	ank Byp	SS			
Residual Lot Capture i	n OSD Tank	93%						
Adjusted SRDs		38	L/s/ha				137	L/s/ha
			OSD C	alculation	ns			
		Extended [Detention	
Basic SSR Volume	Ext Detention Storage		m ³			Total Storage	1192.88	m ³
Total Rainwater Tank	-	0.00	m ³			Ü	0.00	m ³
Storage Volume						Total	1192.88	m ³
Storage Volume	Ext Detention Storage	688.20	m ³			Flood Detention Storage	504.68	m ³
OSD Discharges	Primary Outlet	87.14	L/s			Secondary Outlet	313,29	L/s
RL of Top Water Level	of Storage	18.750	m				19.800	m:
RL of Orifice Centre-lin	e	16,800	m				18.700	m
Number of Orifices		1	-				1	•
Estimated Downstream	Flood Level	15,59	1.5 yr AF	11			18.12	100 yr ARI
Downstream FL - RL o	f Orifice Cente-line	-1.21	Satisfa	ctory		Satisfactory	-0.58	m
Design Head to Orifice	Centre	1.950	m		TWI	L Ext Detn Storage - RL Orifice	0.050	_m
Calculated Orifice Dian	neter	173	mm	Satisfacto	у	Satisfactory	819	mm
		CHECK CONT.						
D. 440.		verflow V	veir & F	reeboar	d Cal	culation		
RL of Minimum Habitat							20.450	m
DI CARLO	Floor Level						20.450	m
RL of Minimum Garage	ir					Parramatta City Council	6.80	m
Length of Overflow We	ir					i arramana city countil	0.75	
Length of Overflow We Site Runoff Coefficient						-		mm/b
Length of Overflow We							206 972.1	mm/h L/s
Length of Overflow We Site Runoff Coefficient Storm Intensity (5 min	100 yr ARI)						206	
Length of Overflow We Site Runoff Coefficient Storm Intensity (5 min Peak Flow over Weir	I 00 yr ARI) ir					Satisfactory	206 972.1	L/s

Only Complete th	is Section	n if a Rainwater Tan	ik Airspace Credit is C	laimed	
The calculations as	sume that th	ie same size rainwater tar	nk is installed on each dwelli	ng	100
				Min	Max
% of Roof draining to Rainwater Tank	0.0%		Satisfactory	0.0%	100%
Total Rainwater Tank Volume	0.00	kL	Minimum 0.0 kL		
Min Volume that triggers Top-up	0.00	kL	Note - Min Vol In Tank <	10% Total T	ank Vol
Total Tank Vol - Min Top-up Vol	0.00	kL			
		Dedicated Airspace			
Dedicated Airspace	0.00	kL	Satisfactory		
	Extended I	Detention		Detention	
Dedicated Airspace Credit	0.00	kL		0.00	kL
Maximum Tank PSD	40	L/s/ha			
Maximum Tank Discharge	0.0	L∕s			
Maximum Head to Centre of Tank Orifice	0.000	m	No Dedicated Airspace		
Calculated Orifice Diameter	0	mm	No Dedicated Airspace		
		Dynamic Airspace			
Maximum Dynamic Storage (Nett Vol)	0.00	kL	Controls minimum % Ro	of to Rainwa	iter Tank
Daily Demand on Rainwater Tank	0.657	kL/d	Satisfactory		
Dynamic Airspace at start of Storm	0.00	kL			
	Extended (Detention		Detention	
Dynamic Airspace Credit	0.00	kL		0.00	kL
Combined Rainwater Tank Credit	0.00	kL		0.00	kL
Maximum Rainwater Tank Credit	0.00	kL		0.00	kL
Rainwater Tank Credit per Dwelling	0.00	kL		0.00	kL
Rainwater Tank Credit for the Site	0.00	m ³		0.00	m ³

Signature:	Date:
3	

On-Site Detention Calculation Sheet for Carctments outside Upper Parramatta River Catchment HED Secondary Outlet

		HE	D 26	condary	Out	let		
Project:	DA OSD4							
Site Address	54-68 FERNDELL	STREET S	OUTH (GRANVILLE				
Job No:	18109							
Designer:	ВВ							
Telephone:	02 9891 5033							
				Site Data				
OSD Area:		Duck Rive	r					
L.G.A		Parramatta	a City Co	ouncil				
Site Area		1.9848	ha	19,848	m ²			
Total Roof Area		1.455	ha		m ²			
Area of Site draining t	o OSD Storage	1.95	ha	19,500	m ²	Satisfactory		
Residual Site Area (Lo	ot Area - Roof Area)	0.530	ha					
Area Bypassing Stora	ge	0.0348	ha					
Area Bypassing / Res	idual Site Area	6.6%				Satisfactory		30% Ma
No. of Dwellings on S	ite	1				Satisfactory		
Site Area per Dwelling	J	1.985	ha					
Roof Area per Dwellin	g	1.455	ha					
		P:	seic O	SD Paran	notors			
		Extended I			i Greis		Detention	
Basic SSR Vols	Ext Detention Storage		m ³ /ha	41		Total Storage	520	m³/ha
Basic SRDs	Primary Outle		L/s/ha			Secondary Outlet	150	L/s/ha
	· ·····z··y outs		Doma			Occordary Codec	100	Lisina
			OSD 1	Tank Byp	ass			
Residual Lot Capture	in OSD Tank	93%						
Adjusted SRDs		38	L/s/ha				137	L/s/ha
			osni	Calculation	nne			
		Extended [7110		Detention	
Basic SSR Volume	Ext Detention Storage		m ³			Tolal Storage	1032.10	m ³
Total Rainwater Tank		0.00	m ³			rotal Storage	0.00	m ³
Storage Volume	0.00	0.00				Total	1032,10	m ³
Storage Volume	Ext Detention Storage	595.44	m ³			Flood Detention Storage	436.66	m ³
OSD Discharges	Primary Outlet		L/s			Secondary Outlet	271.65	L/s
		10110				obderidary output	211.00	Lis
RL of Top Water Level	of Storage	18.200	m				19.500	m
RL of Orifice Centre-lin	ne	16.000	m				17.000	m-,-,
Number of Orifices	İ	1	-			1	1	<u>-</u>
Estimated Downstream	n Flood Level	15.52	1,5 yr A	RI			16.96	100 yr ARI
Downstream FL - RL o	f Orifice Cente-line	-0.48	Satisf			Satisfactory	-0.04	m
			m		TWI	L Ext Detn Storage - RL Orifice	1.200	m
Design Head to Orifice	Centre	2,200	m					
		156	mm	Satisfacto	ry	Satisfactory	345	mm
Design Head to Orifice	neter	156	mm			•	345	mm
Design Head to Orifice Calculated Orifice Diar	neter		mm			•		mm
Design Head to Orifice Calculated Orifice Diar RL of Minimum Habital	neter O ble Floor Level	156	mm			•	20.450	mm
Design Head to Orifice Calculated Orifice Diar RL of Minimum Habital RL of Minimum Garage	neter O ble Floor Level a Floor Level	156	mm			•	20.450 20.450	m m
Design Head to Orifice Calculated Orifice Diar RL of Minimum Habital RL of Minimum Garage Length of Overflow We	neter O ble Floor Level e Floor Level ir	156	mm			culation	20.450 20.450 6.00	m
Design Head to Orifice Calculated Orifice Diar RL of Minimum Habital RL of Minimum Garage Length of Overflow We Site Runoff Coefficient	neter O ble Floor Level e Floor Level ir	156	mm			•	20.450 20.450 6.00 0.75	m m m
Design Head to Orifice Calculated Orifice Diar RL of Minimum Habital RL of Minimum Garage Length of Overflow We	neter O ble Floor Level e Floor Level ir	156	mm			culation	20.450 20.450 6.00 0.75 206	m m m
Design Head to Orifice Calculated Orifice Diar RL of Minimum Habital RL of Minimum Garage Length of Overflow We Site Runoff Coefficient Storm Intensity (5 min Peak Flow over Weir	oble Floor Level e Floor Level ir 100 yr ARI)	156	mm			culation	20.450 20.450 6.00 0.75 206 836.9	m m m mm/n
Design Head to Orifice Calculated Orifice Diar RL of Minimum Habital RL of Minimum Garage Length of Overflow We Site Runoff Coefficient Storm Intensity (5 min	ble Floor Level e Floor Level ir 100 yr ARI)	156	mm			culation	20.450 20.450 6.00 0.75 206	m m m

The calculations a	ssume that th	ne same size rainwater ta	ank is installed on each dwellin	Min	Max
% of Roof draining to Rainwater Tank	0.0%		Satisfactory	0.0%	Max 100%
Total Rainwater Tank Volume	0.00	kL	Minimum 0.0 kL		
Min Volume that triggers Top-up	0.00	kL	Note - Min Vol in Tank < 1	0% Total Ta	nk Voi
Total Tank Vol - Min Top-up Vol	0.00	kL			
		Dedicated Airspace			
Dedicated Airspace	0.00	kL	Satisfactory		
·	Extended l	Detention		Detention	
Dedicated Airspace Credit	0.00	kL		0.00	kL
Maximum Tank PSD	40	∐s/ha			
Maximum Tank Discharge	0.0	U s			
Maximum Head to Centre of Tank Orifice	0.000	m	No Dedicated Airspace		
Calculated Orifice Diameter	0	mm	No Dedicated Airspace		
		Dynamic Airspace			
Maximum Dynamic Storage (Nett Vol)	0.00	kL	Controls minimum % Roo	f to Rainwa	ter Tank
Daily Demand on Rainwater Tank	0.657	kL/d	Satisfactory		
Dynamic Airspace at start of Storm	0.00	kL			
	Extended (Detention		Detention	
Dynamic Airspace Credit	0.00	kL		0.00	kL
Combined Rainwater Tank Credit	0.00	kL		0.00	kL
Maximum Rainwater Tank Credit	0.00	kL		0.00	kL
Rainwater Tank Credit per Dwelling	0.00	kL		0.00	kL
Rainwater Tank Credit for the Site	0.00	m ³		0.00	m ³

Signature:	Date:



Appendix C – UPRCT OSD Checklist

I.1 OSD Stormwater Concept Plan Submission – Short Form

This form is to be completed by the stormwater designer and submitted to Council together with the plan(s), any necessary attachments and a completed OSD Calculation Sheet.

PR	OJECT ADDRESS: 54-68 Ferndell Street, South Granville		
Со	SD DESIGNER DETAILS: Sparks+Partners Consulting Engineers mpany Name:		
Ad	dress: _Level 1, 91 George Street, Parramatta		
	ephone No.: 02 9891 5033 Fax No: N/A		
Ac	creditation Organisation: <u>EA</u> Accreditation Reference: _	NER 3300498	
Na	me of designer: Benjamin Barrett Date: 31/05/2018		
_	(Print Name)		
Co	uncil Reviewer's Name; Date:	1	
1.	CHECKLIST Flooding:	Complies to Handbook Yes No	Council Agrees
•	Is the site (whole or partly) below the 100 year ARI flood level?	7 D	Yes No
	If yes, does the OSD system reflect the flood affectation?		
	Have floodplain issues been addressed (eg storage, obstructed flow etc)?		
2.	External catchment: (refer Section 4.1.3)	₫ □	пп
	Is there an external catchment draining into the site?		
	If Yes, have calculations of 100 year ARI flow been submitted & full area of catchment shown?		
3.	OSD Storage : (refer Sections 4.1.4, 4.1.5 & 6.5)		
	Was the storage volume calculated using the UPRCT Calculation Sheet?		
	Is the area bypassing the OSD storage less than 30% of the residual area?	J -	
	Is there free discharge at the outlet or provision made for a drowned outlet?		
	Has the storage been located at the lowest point of the site to collect surface and roof gutter overflow		
4.	Site information:		ЦЦ
	Has the following information has been shown on the plans:		
	- scaled site layout showing all buildings, roadways and landscaped areas		
	- spot levels and contours (including adjoining properties)		
	- location, dimensions and extent of detention storages		
	- location of any floodways or flowpaths through the site	I√ L	
	 location of any other constraints, e.g. easements, sewer and other services or Water Sensitive Urban Design (WSUD) features 		
5.	OSD Calculation Sheet is attached	■	шШ



Appendix D – Maintenance & Monitoring Schedule

Date:



Author Name & Signature:

54-68 Ferndell Street South Granville

General Notes:
1 - Maintenance is to be carried out with regard to relevant occupational health and safety guidelines and standards. This includes all confined space, traffic management, fall arrest and other requirments.
2 - Initial monitoring and inspections of the stormwater system post commissioning are to be carried out every 3 months for the first year of operation. The amount and type of debtine is to be noted and recorded. This information shall be used to determine if modification of the frequency of inspections is required.

3 - The frequency of inspections shown in the stormwater maintenance schedule are the maximum periods. Inspection frequencies may be recluced upon completion of the initial monitoring and inspection program as noted in note 2.
4 - Blank copies of the maintenance schedule are to be made and filled out during each subsequent inspection with the details kept on site for future reference.

Date of Next Inspection Date of Inspection: Inspected by:

Rem to be inspected	Frequency	Frequency Performed by:	hapected	Profession Required	Mainterance Procedure Committerance	Hance
Operation of the Party of the P			Ye. No	Yearling		ate
	Six Monthly/	Owner /				
Eaves/Box Guttering System and Downpipes	After Major	Maintenance			Inspect and remove any build up of sediment, debris, litter and vegetation within gutter system,	
	Storm	Contractor				
	Four Monthly/	Owner /			Perrow grate and inspect internal walls and base, repair where required. Perrowe any collected sectiment, debris, litter and wegetation, (e.g., Vacuny eductor truck) Inspect and ensure grate is clear of	
oronnwater son ace met and junction pits	Arrer Major Storm	Confractor			recimient, debris, inter and vegetation. Ensure flush placement of grate on reformant	
		Owner /				T
drainage system (that's visible)	Bi-annually	Maintenance			mapset all d'ainage structures noting any dilapidation, carry out required repars.	
Harawater Tarik						
		Owner/				
First Flush Device	6 Monthly	Maintenance			instituta devoe to ensure correct operation. Remove accumulated litter & debrits, if device is not functioning properly repair or replace.	
		Contractor			10 minutes	
		Owner/				
Internal Inspection	6 Monthly	Maintenance			and the state of access by an interest including the presence of musquito larvae, in present, locktiff access by an interest of access by a contract of access by a	
		Contractor			enay.	
		Owner/				
Tank and lank roof	6 Monthly	Maintenance			the structural integrity of tank including roof and access covers. Any diapidation including holes or gaps are to be noted and repaired.	
		Contractor				
WEST-D Membranes						
	Refer	Maintenance /				
Ocean Protect StormFilter Cartridges	Manufactures	Specialised			Helev to manufacturers operation and maintenance manual.	_
	Manual	Contractor				
	Refer	Maintenance /			TO A SECTION OF THE PROPERTY O	
Deean Protect Enviropods	Manufactures	Specialised			Peter to manufacturers operation and maintenance manual.	
	Manual	Contractor				
CSD and Stormfilter Tarks						
	Six Monthly/	Owner/				
Trash Screen	After Major	Maintenance			Inspect van screen to ensure correct operation. Remove accumulated litter & debris, If device is not functioning properly repair or replace.	
	Statm	Contractor			The state of the s	
		Owner /				
Fank and tank roof	6 Monthly	Maintenance			Check structural inegrity of tank including roof and access covers, Any dilapidation including holes or gaps are to be noted and repaired.	
		Contractor				